## **Attachment A11**

**Structural Statement** 



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## Project reference: CANVA – 8-24 Kippax St, Surry Hills - Project No:131006 – Planning Proposal

Dear Stephen,

Meinhardt (NSW) Pty Ltd have been engaged by Canva for the structural design documentation of the proposed redevelopment of the abovementioned project.

The existing building is a 10 storey reinforced concrete building with two levels of basement. The building is believed to be founded on extremely low rock. The existing footings are composed of piled and pad footings as indicated on the existing drawings. This is based on the desk top geotechnical study undertaken for the site but is to be confirmed.

The proposed structural alteration and strengthening works to the building include, but are not limited to:

- Demolition of the existing stair & lift cores and shear walls. The construction of a relocated lift and stair core within the building;
- Relocation of the existing substation;
- 2No new levels on the roof which include commercial office floor, rooftop terrace and plant space;
- Steel transfer truss at Level 01'
- Strengthening of the existing concrete columns via new reinforced concrete (RC) "jacketing";
- Additional steel K-bracing. Together with the new lift and stair core, these will provide overall lateral stability to the building;
- New RC and steel columns to support the slabs for the revised architectural layouts. These will be founded on new pad and piled foundations;
- Strengthening of the existing footings;
- A new "Town Hall" stair way linking the Kippax St and Sophia Lane streetscapes.

Meinhardt have assessed the existing building and the proposed structural development works as indicated on the architectural plans and we confirm that the building will be designed in accordance with the relevant standards such that the existing structure together with the new structural elements will be able to support the proposed new loads.

Our design relies upon the acceptance of the list of non-compliance issues and assumptions as discussed below.

Our assessment of the existing structure is based on the following information provided to us:

- Existing structural drawings by V.A. Lamaro dated 1969
- Existing architectural drawings by L.A. Kubany dated 1969
- "Report on Concrete Testing Investigation 8-24 Kippax St, Surry Hills" by Mahaffey Associates dated 23 October 2023 (Ref: 20475)



The structural design of the new structural elements will comply with the Structural Provisions of Part B1 (Volume 1) of the Building Code of Australia, subject to the list of non-compliance issues and assumptions discussed below.

In addition, the capacity of the existing structure will be assessed such that the additional loads and/or alterations will not cause a decrease in its structural performance in comparison to the original design.

## List Of Non-Compliances and Assumptions

It is noted that the existing structure was constructed in 1969. The building codes and standards of that time would typically have been used as a basis for the design of the existing structure by the original structural designer.

There will be requirements in the current day codes that the existing structure will not have been designed for nor will be able to comply with now.

In addition, there are assumptions that we have made in our design to date.

A list of these items is included in Appendix A at the rear of this letter. It should be noted that this list is not exhaustive and may be added to as the structural design develops.

Yours sincerely,

Neal Fage

Neal Foye Associate Director

Meinhardt (NSW) Pty Ltd E: <u>Neal.Foye@meinhardtgroup.com</u>



Appendix 1

– List of Structural Non-Compliances and Assumptions List

Project No: 131006

27/11/2023



## LIST OF CODE NON-COMPLIANCES AND ASSUMPTIONS ADOPTED IN DESIGN

F			-		
	No: Non-compliance / Assu	ation Comments	Current Day Standards/ Code	e Method for Compliance/Assumption Adopted	
-1			Reference	Methodology	Notes
	1 Pounding	Requirement to settack adjacent building between each other such that there is no contact between the buildings in the every of an earthquake Given that 2-4 Kopas 31 and the neighbouring 25 Kopas 31 are existing building, it is unlively that the two till have sufficient clearance betwee each other to satisfy this clause. This will need to be confirmed via investigation through local removal of the boundary wall 0-24 Kipps 31 are each other to satisfy this clause. This will need to be confirmed via investigation through local removal of the boundary wall 0-24 Kipps 31. Note the this lam is made on the accurption that there is little or or go between the existing building. This will have to be confirmed by investigation of the existing building. Namely local removal of the existing macrony along the party wall. Our preliminary structural analysis is th a gap between the existing buildings of 40mm of more would be required to comply with code requirement. If this gap (id not existing, the atternative paths of dispensation or significant remedial works would then apply.	AS 1170.4 n Clause 5.4.5 at	Dispensation of the clause by PCA and Council	Given that this is an existing condition which is very difficult to rectify and which is an interent condition that will we sint in early all existing buildings with the City of Sydney, a dispensation of the clause may be acceptable to the PCA and Council.
	<ol> <li>Adoption of existing structur. for use as a basis of design redevelopment</li> </ol>	I Existing drawings dated 1969 and 1970 have been obtained from City of Sydney Council archives. Meinhardt intend to use these drawings as a basis of the existing structure to be incorporated in the design of the redevelopment.	n/a	An investigation of the easily concrete structure will be undertaken on alle by a concrete testing company. This meetingation will holds: taking approximate concrete cores to testing, scanning of existing reinforcement and measurement of existing structural elements on site. A cross check of the results of the investigation report tigainst the existing structural drawings will be undertaken in order to provide a level of confidence that the existing structural drawings we subably reliable for this use.	It should be noted that the concrete testing will be undertaken at a number of locations and will be a reasonable sample expresentation. It is not interded to lots tervery structural element or every assumed concrete powr which would have been poured at the time of construction. It is believed that taking such a nample investigation is a reasonable approach to provide a sufficient level of comfort to allow the use of the existing drawing and the same of the setting drawing and the same of the same structure.
H					
	3 Geotecnnical investigation	Existing drawings and/or two and 10/0 make been colonized from Lity of sydney Jounci and new from the drawings instance a mix of pield and pi rootings for the existing building, pad and piled foundations to support the proposed building loads including an increase in loads in some cases. Strengthening of the foundations will be undertaken where required. It is intended to use the foundations noted on the existing structural drawings as a basis of the design for the redevelopment.	ka nva	It is intended to undertake sample investigation of the easting pair locing's and pier documations by a geotechnical investigation company. The investigation adjacent to a sample number of existing pad foundations to confirm their size, depth and founding strata, available adjacent to a sample number of piles and use magnetic gradiometry testing (or other similar methodology) to confirm the depth of those existing piles. Once the sample testing piles will be undertaken by drilling boreholes to a sample number of piles and use magnetic gradiometry testing (or other similar methodology) to confirm the depth of those existing piles. Once the sample testing of existing pad and pile information is confirmed, this will be cross checked against the existing drawings in order to provide a level of confidence that the existing drawings are suitable to be used as a basis of the design.	The even of restrict to be funderaken will be as required by the geoechnical to achieve a reasonable level of confidence in the existing structural drawings and investigation findings on site.
				An assessment of the capacity of the existing piles and pad footings will then be undertaken by the geotechnical engineer and their performance under the proposed increased loads assessed.	
	4 Ongoing Durability & Design existing structure	e of the Compliance to AS 3800 provides a design life of 50 years (++ 20%) for new structures. Any existing structure is at the end of its design life and AS 3800 section 4 for durability and design life will not apply.	AS 3600 Section 4	I the concrete investigation of the building will include sample testing of the concrete structure for chloride ingregas, reinforcement corrorsion, et (one commonly given the universitel areference of "concrete cance". Where any such defects are found in the building, rectification works as advised by the concrete testing specialist will be undertaken.	Is is intended that the concrete testing report, none complete, will be able to state that there will be no significant indi or interforcement correction or concrete deterioration for a further 44-50 years once any advised rectification works (If required and as stated in the concrete testing report) are understane and provided reasonable maintenance of the existing building is provided during that period.
	<ol> <li>Restraint of longitudinal rein in existing concrete columns</li> </ol>	Horizontal column ties are required to restrain main vertical column reinforcement. These prevent the vertical main bars from breaking out of th     column under high bar stresses.     While the existing structural drawings indicate that ties have been provided in the existing concrete columns, the tie diameters and spacing may     not comply with current day codes	AS 3600 Clause 10.7.4	For the restrain of main vertical column bars in existing columns is required, an assessment of the restraint bars, forces and lies provided as noted on the structural drawings will be undertaken. These will be assessed from first principles of the stresses in the columns in order to provide a performance solution compliance rather than a deemed to comply approach as provided in AS 3600.	Where restraint stresses are found to exceed the lie capacities, strengthening works to the existing columns will be documented.
Ŀ	6 Restraint of longitudinal rein	cement Horizontal column ties are required to restrain main vertical column reinforcement. These prevent the vertical main bars from breaking out of the	AS 3600	An assessment of the restrain forces required for the main bars will be undertaken	n/a
	in proposed concrete column strengthening "jacketing"	column under high bar stresses. Internal lies will not be able to be provided in column "jacketing" due to the existing column being in place. Hence the deemed to comply requirement of the provision for relating lies can not be achieved.	Clause 10.7.4	Note that downleid into the existing columns with chemical anchors will be provided to restrain these bars in this condition. Hence a performance solution compliance will be provided rather than a deemed to comply approach as provided in AS 3600.	
H	7 Ductility of the existing roinfr	ement The durblity of the existing reinforcement may not be compliant with the adjacent code clauses	AS 3600 Claura	Testing will be undertaken to confirm the ductility of the existing reinforcement	n/a
			1.1.2 and clause 3.2.1	This testing may find that the existing reinforcement is complaint to AS 3000 with respect to ductility. If the testing during the existing reinforcement does not comply in this respect, a dispensation of these clauses will be required.	
Ī	8 Deflection	Measured deflections in the existing slabs may exceed the "guidelines for serviceability limit states" provided in the codes	AS 1170.0 Appendix C	A visual inspection of the existing building will be undertakan once all existing intraemal finishes are removed. An assessment of the slab deflections provided in the building topographocal survey will be reviewed also. A comparison will then be made with suggested deflection limits provided in codes e.g. AS 1170.0	If excessive deflections are discovered in the existing slabs, strengthening works can be provided.
t	9 Detailing of the reinforcement	n the Flat plates require certain deemed to comply detailing of lengths and spacing of bars in the code.	AS 3600	A finite element (FE) computer analysis of the existing slabs will be undertaken to confirm if the reinforcement as	Where existing reinforcement is found to be insufficient, for minimum strength requirements
	existing flat plate slabs	The existing slabs will not comply with this in many locations.	Clause 9.1.2 & Clause 9.1.3	noted on the existing structural drawings is sufficient as per the outputs of the analysis. Hence a performance solution is proposed in lieu of the deemed to comply approach of the code.	strengthening of the slab will be provided.
1	10 Structural integrity reinforcer	nt Bottom reinforcement is required in the code in order to increase the resistance of progressive collapse of the slabs at its connections to	AS 3600	A dispensation of this clause is required as there is no alternative solution method of achieving this within the	n/a
		columns. It is noted that his requirement was not included in the concrete code until 2018. Structural integrity reinforcement was not required before this. The clause requires a minimum amount of bottom reinforcement be provided within a flat plate lab and for this reinforcement to be continuous over the supporting column. This is not how reinforcement in slabs was typically detailed in Australia prior to 2018 and this is not what is provid in the existing slabs at 8-24 Kippax St as shown on the existing drawings.	Clause 9.2	existing atlass in order to comply with the deemed to comply clause. The lustification of any dependation would rely on the fact that this is a misihely new clause that was not negate prior to a new revision of AS 3600 in 2018. The clause is intended as a back up to punching shear failure of the alabs. Punching shear failure of the existing slabs will still be checked to AS 3600 Clause 9.3 and strengthening of the alabs provided where required.	8

List of Codes: AS 11700 - Structural Design Actions - Part 0 : General Principles AS 11704 - Structural Design Actions - Part 4 : Earthquake Actions in Australia AS 3600 - Concrete Structures